

Analysis of Runoff Coefficient Value on Retention Ponds in Flores Island

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Abstract. Flores Island is one of four big islands in NTT province with an area $\pm 13,540$ km² divided into 8 districts. The area is included in areas with unequal distribution of rainfall. Therefore, the amount of water availability during the dry season is relatively low then require to attempts of rainwater harvesting. One of the alternatives is by building a retention pond. The important parameter in the calculation of water availability is the value of runoff coefficient. The purpose of this research is to invent the runoff coefficient value of 30 retention ponds in 8 districts of Flores Island. In this study use rainfall data, climatology and technical of retention basin for the analysis of run off coefficient. The analysis method uses the Penman modification for evapotranspiration calculation and method F.J. Mock for discharge calculation. The result in graphical model uses monthly rainfall data and land slope data. Based on the analytical calculation method, the value of run off coefficient for each district in Flores was ranging 0.00 - 0.72. The minimum value of runoff coefficient happened in November was ranging from 0.00 - 0.39, and the maximum value of runoff coefficient happened in January was ranging from 0.48 - 0.72.

Keywords: retention ponds; evapotranspiration; runoff coefficient

1. Introduction

Flores Island is a region that has different climatological conditions which the western Flores is wetter than eastern parts. One of the effective strategies in resolving the lack of water supply in semi-arid area, that is with harvesting rainwater as build retention pond or retention basin. The important parameter to calculate of water availability at retention ponds is the coefficient of runoff. The value of runoff coefficient is a number that shows the ratio of surface runoff occurs against the volume of rainfall in a region. Runoff coefficient value needed to calculate water discharge inflow to the retention ponds.

One way to resolve the problem of lack the water availability is building the retention ponds construction. Retention ponds or usually called “embung” serves as a place to store the excess water during the rainy season. The excess of rainwater is stored by "embung" can be utilized during the dry season to fulfil the requirement of water for the population, cattle, and garden. Besides the rainwater falls on the surface to the retention ponds, some of the water also comes from the surface runoff [9].

Run off from catchment must be adequate to meet requirements. In practise, the catchment should be neither too small nor too large [5]. First, it will not meet the requirements, and second, the rapid sedimentation caused by the silt carried by the stream. The loss in storage capacity is attributed to the accumulation of sediment in the retention ponds [4].

Surface runoff occurs when the rainfall is greater than the ability of the soil to absorb water. So that the excess water flows down to the river, lake, or sea. One indicator that determines the amount of surface runoff is the value of runoff coefficient. The runoff coefficient is mentioned "high" when it close to 1.00, which indicates that more rainwater becomes the surface runoff. But if the rainwater percolate as infiltration to underground then the runoff coefficient is close to 0.



Figure 1. The retention pond in Larantuka – Flores Island

Water availability of a retention ponds are obtained from the discharge entering the catchment area due to the high intensity rainfall. Those discharge influences the value of runoff coefficient that one of important factor in water conservation for planning of retention pond development. The value of the runoff coefficient in an area also variety depends on the meteorological conditions and the land cover conditions of the watershed [8].

In this study use rainfall data, climatology and technical of retention basin for the analysis of run off coefficient. The analysis method uses the Penman modification for evapotranspiration calculation and method F.J. Mock for discharge calculation. Evapotranspiration (E_p) is the combination of two separate processes, where liquid water is converted to water vapour (vaporization) from the soil, wet vegetation, open water or other surfaces, as well as from plants by transpiration through stomata [1]. Potential evapotranspiration is evaporation that affected by climatic conditions. The main weather variables affecting E_p are temperature, solar radiation, wind speed and vapour pressure. The calculation of evapotranspiration method uses the Penman Modification method and can be formulated as follows:

$$E_p = c[w \times R_n + (1 - W) \times f(u) \times (e_a - e_d)] \quad (1)$$

whereas:

$$E_p = \text{potential evapotranspiration (mm/month)}$$

- c = adjustment factor to compensate for the day and night weather effects
w = temperature and elevation related weighting factor for the effect of radiation on E_p
 R_n = net radiation ($R_{ns} - R_{ni}$) in mm/day
 $(1-w)$ = a temperature and elevation related weighting factor and the effect of wind humidity on E_p
 $f(u)$ = a wind (km/day) related function = $0.27 (1 + (U/100))$
 e_a = saturation vapor pressure at the mean air temperature in degrees centigrade (mbar)
 e_d = mean actual vapor pressure of air in mbar = $e_a (RH/100)$
RH = relative humidity

The calculate of dependable flow by FJ Mock method is a calculation method based on rainfall data, evapotranspiration, and local hydrological characteristics. The calculation criteria of simulation FJ Mock method include analysis:

- (a) Rainfall data,
(b) Limited evapotranspiration,

$$E_t = E_p - E \quad (2)$$
(c) Water balance,

$$\Delta S = P - E_t \quad (3)$$

$$WS = \Delta S - SS \text{ and } WS = 0 \text{ if } \Delta S < SS \quad (4)$$

$$WS = 0 \text{ if } \Delta S < SS \quad (5)$$
(d) Runoff and groundwater storage,

$$\Delta V_n = V_n - V_{(n-1)} \quad (6)$$

$$V_n = k V_{(n-1)} + 0.5(1+k) I \quad (7)$$

$$I = WS \times I_n \quad (8)$$

whereas:

- E_t = limited evapotranspiration,
 E_p = potential evapotranspiration,
 E = the difference between E_t and E_p (mm)
 WS = excess water (mm/month)
 ΔS = effective rainwater (mm/month)
if $\Delta S < 0$, then the surplus water = 0
 SS = groundwater content (mm)
 ΔV_n = deviation of groundwater storage volume (mm/month)
 V_n = volume of ground water deposits month n (mm/month)
 K = qt/q_0 = soil recession factor (assumed 0 - 1)
 V_{n-1} = volume of ground water savings month (n-1) (mm/month)
 I = volume of ground water savings month n (mm/month)
 I_n = infiltration coefficient (assumed 0 - 1)
- (e) Dependable flow, consist of [11]:
Base flow (BF) = infiltration (I) – deviation the volume of ground water (ΔV_n)
Direct runoff (DR) = excess water (WS) - infiltration (I)
Flow = Base flow (BF) + Direct runoff (DR)
- (f) Runoff Coefficient
The runoff coefficient is defined as the ratio between the peak of surface runoff to the intensity of rainfall. This factor is the most variable that determine the calculation results of flood discharge. The main factors affecting the runoff coefficient are the rate of soil infiltration or percentage of impermeable land, land slope, land covering and intensity of rainfall [10].
In the calculation of surface runoff coefficient analysis used with analytical method as follows [2]:
– Count the average rainfall in catchment area at the given time (t), e.g. $P = \text{mm/month}$,

Methods carried out with: a) descriptive study, i.e. by doing literature study and collecting of secondary data such as topographic maps, precipitation data, climate data and land use of the watershed; b) qualitative research, i.e. from descriptive studies were transferred in the form of charts to be used as a reference in the calculation to criteria design of retention ponds. Flow chart of the method shown in Figure 3.

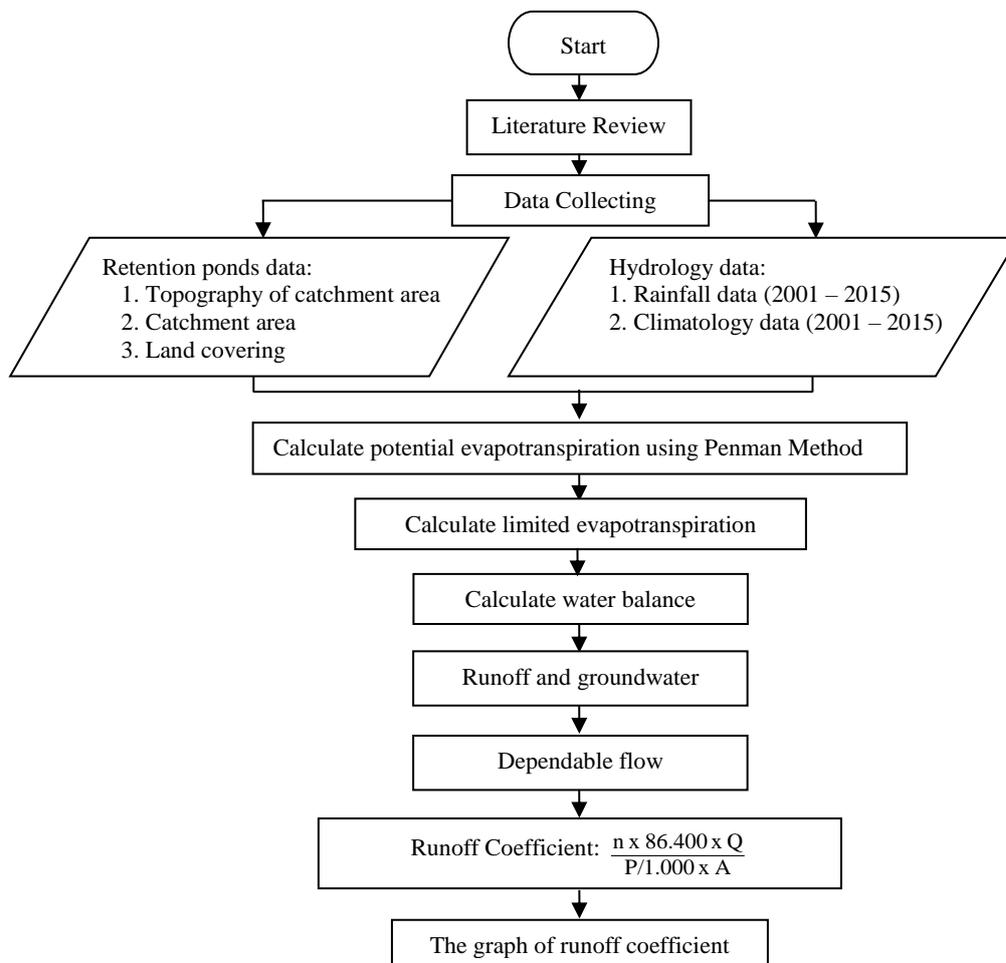


Figure 3. Flow chart of the research [6]

3. Result and Discussion

The value of surface runoff coefficient is a number that shows the ratio of the amount of surface flow that occurs as a result of the amount of rainfall that falls in a region against the volume of rainfall. The coefficient values are generally different in each region, depends on the permeability and the soil ability to keep water.

Data of monthly rainfall taken from rainfall stations which expected to represent phenomena of rainfall that occurred retention ponds location in Flores Island. The calculate of runoff coefficient with analytical and uses graph of Research and Development Centre for Water Resources or called 'Puslitbang' [7] in January shown in Table 1.

Table 1. The value of runoff coefficient on 30 retention ponds in Flores Island in January

District	Retention Ponds	Slope of land (m/km)	Monthly Rainfall (mm)	Runoff coefficient	
				Analytical	Graph
Sikka	Waer Koja	< 100	181.50	0.26	0.04
Sikka	Mageweda	< 100	144.56	0.37	0.01
Sikka	Napunggelang	< 100	144.56	0.37	0.01
Sikka	Habiheret	< 100	181.50	0.26	0.04
Ende	Korangawe	< 100	289.53	0.65	0.22
Ende	Aenangge	< 100	289.53	0.60	0.48
Ende	Tana Merah	< 100	289.53	0.58	0.22
Ende	Tubu Bewa	< 100	253.80	0.55	0.40
Nagakeo	Nunu Beza	< 100	472.20	0.68	0.62
Nagakeo	Kelimeli	< 100	472.20	0.67	0.62
Nagakeo	Robo Alo	< 100	472.20	0.67	0.62
West Manggarai	Joneng	< 100	220.20	0.34	0.10
East Manggarai	Waekao	< 100	160.07	0.18	0.04
East Manggarai	Komba	< 100	160.07	0.17	0.04
Ngada	Ndoraliti	< 100	525.87	0.76	0.63
Ngada	Budhai	< 100	525.87	0.76	0.63
Ngada	Tanah Ewer	< 100	525.87	0.76	0.63
East Flores	Belohuko	100 - 200	372.40	0.54	0.52
East Flores	Angi Marak	100 - 200	372.40	0.54	0.52
East Flores	Gajak Leda	100 - 200	372.40	0.53	0.61
East Flores	Sabu Leti	100 - 200	372.40	0.52	0.61
West Manggarai	Nara I	100 - 200	220.20	0.36	0.34
West Manggarai	Waecere	100 - 200	220.20	0.35	0.34
East Manggarai	Kempo	100 - 200	160.07	0.19	0.15
East Manggarai	Hedok	100 - 200	160.07	0.17	0.15
Manggarai	Kondamari	100 - 200	427.40	0.73	0.63
Ngada	Ramba I	100 - 200	525.87	0.75	0.63
Ngada	Hoboremangai	100 - 200	525.87	0.76	0.63
Ngada	Ngara I	100 - 200	525.87	0.76	0.63
Manggarai	Pedang	> 200	427.40	0.72	0.81

Based on the analysis data in Table 1, the value of runoff coefficient between analytical method and by using 'Puslitbang' graph on 30 retention ponds have differences of about 23.52%. Only three districts have a considerable difference i.e. Sikka, Ende, and in part of East Manggarai.

For the overall calculation of surface runoff coefficient values for each district in Flores Island can be seen in Table 2.

Table 2. Recapitulation of the value of runoff coefficient on 30 retention ponds in Flores Island

Name of retention ponds	Month											
	Jan	Feb	March	Apr	May	Jun	Jul	Aug	Sept	Oct	Nov	Dec
Belohuko	0.538	0.554	0.472	0.432	0.283	0.952	0.107	0.081	0.004	0.000	0.000	0.274
Angi Marak	0.536	0.540	0.472	0.431	0.282	0.948	0.106	0.080	0.004	0.000	0.000	0.286
Gajak Leda	0.533	0.549	0.467	0.426	0.276	0.935	0.105	0.079	0.003	0.000	0.000	0.269
Sabu Leti	0.523	0.539	0.458	0.413	0.261	0.898	0.101	0.076	0.003	0.000	0.000	0.259
Waer Koja	0.264	0.314	0.262	0.333	0.367	0.126	0.574	0.010	0.002	0.000	0.012	0.239
Mageweda	0.372	0.373	0.418	0.451	0.413	0.340	0.089	0.000	0.004	0.001	0.000	0.201
Napunggelang	0.373	0.375	0.420	0.453	0.415	0.342	0.089	0.000	0.004	0.001	0.000	0.201
Habiheret	0.261	0.324	0.279	0.350	0.380	0.131	0.120	0.010	0.002	0.000	0.019	0.251
Korangawe	0.652	0.643	0.598	0.511	0.417	0.282	0.086	0.055	0.304	0.104	0.031	0.477
Aenangge	0.599	0.574	0.531	0.413	0.292	0.168	0.056	0.035	0.261	0.054	0.004	0.399
Tana Merah	0.577	0.551	0.511	0.383	0.255	0.133	0.046	0.030	0.248	0.039	0.003	0.382
Tubu Bewa	0.554	0.611	0.551	0.533	0.434	0.160	0.102	0.040	0.533	0.273	0.030	0.445
Nunu Beza	0.680	0.645	0.542	0.548	0.374	0.482	0.386	0.228	0.208	0.153	0.383	0.633
Kelimeli	0.675	0.637	0.530	0.537	0.361	0.475	0.368	0.222	0.200	0.146	0.375	0.629
Robo Alo	0.674	0.637	0.530	0.536	0.360	0.475	0.367	0.221	0.199	0.145	0.375	0.628
Budhai	0.756	0.780	0.768	0.713	0.627	0.601	0.459	0.337	0.281	0.184	0.580	0.685
Hoboremangai	0.756	0.781	0.769	0.714	0.628	0.602	0.460	0.339	0.282	0.185	0.580	0.685
Ndoraliti	0.760	0.785	0.774	0.720	0.636	0.606	0.461	0.347	0.285	0.191	0.585	0.688
Ngara I	0.761	0.786	0.777	0.722	0.640	0.608	0.462	0.351	0.287	0.194	0.587	0.690
Ramba I	0.754	0.777	0.759	0.699	0.598	0.588	0.426	0.301	0.278	0.162	0.569	0.682
Tanah Ewer	0.760	0.784	0.774	0.720	0.636	0.606	0.461	0.347	0.285	0.191	0.585	0.688
Hedok	0.168	0.218	0.209	0.164	0.050	0.037	0.170	0.060	0.013	0.006	0.148	0.175
Kempo	0.192	0.246	0.238	0.187	0.057	0.041	0.170	0.061	0.013	0.020	0.161	0.197
Komba	0.172	0.223	0.214	0.169	0.052	0.038	0.170	0.060	0.013	0.009	0.151	0.179
Waekao	0.175	0.227	0.218	0.172	0.052	0.038	0.170	0.060	0.013	0.011	0.152	0.182
Kondamari	0.731	0.781	0.759	0.823	0.779	0.651	0.409	0.225	0.354	0.409	0.639	0.707
Pedang	0.725	0.776	0.754	0.818	0.771	0.639	0.404	0.220	0.346	0.400	0.633	0.702
Embung Joneng	0.343	0.212	0.228	0.233	0.152	0.077	0.037	0.089	0.011	0.002	0.000	0.197
Nara I	0.355	0.230	0.241	0.241	0.166	0.082	0.039	0.102	0.013	0.002	0.000	0.211
Waecere	0.346	0.216	0.231	0.235	0.155	0.079	0.037	0.092	0.012	0.002	0.000	0.200
Maximum	0.761	0.786	0.777	0.823	0.779	0.952	0.574	0.351	0.533	0.409	0.639	0.707
Minimum	0.168	0.212	0.209	0.164	0.050	0.037	0.037	0.000	0.002	0.000	0.000	0.175
Average	0.519	0.523	0.492	0.469	0.372	0.405	0.235	0.139	0.149	0.096	0.220	0.415

Table 2 shows that the value of runoff coefficient in December until April have a greater value than another month. This is because the rainfall is quite high and lead to greater runoff. Thus, the filling of water ponds fulfilled in that months. In August – November, almost of all the retention ponds weren't receiving runoff from upstream due to dry season in Flores island.

For the graph of runoff coefficient against high of monthly rainfall and slope of the land in January - April shown in Figure 3.

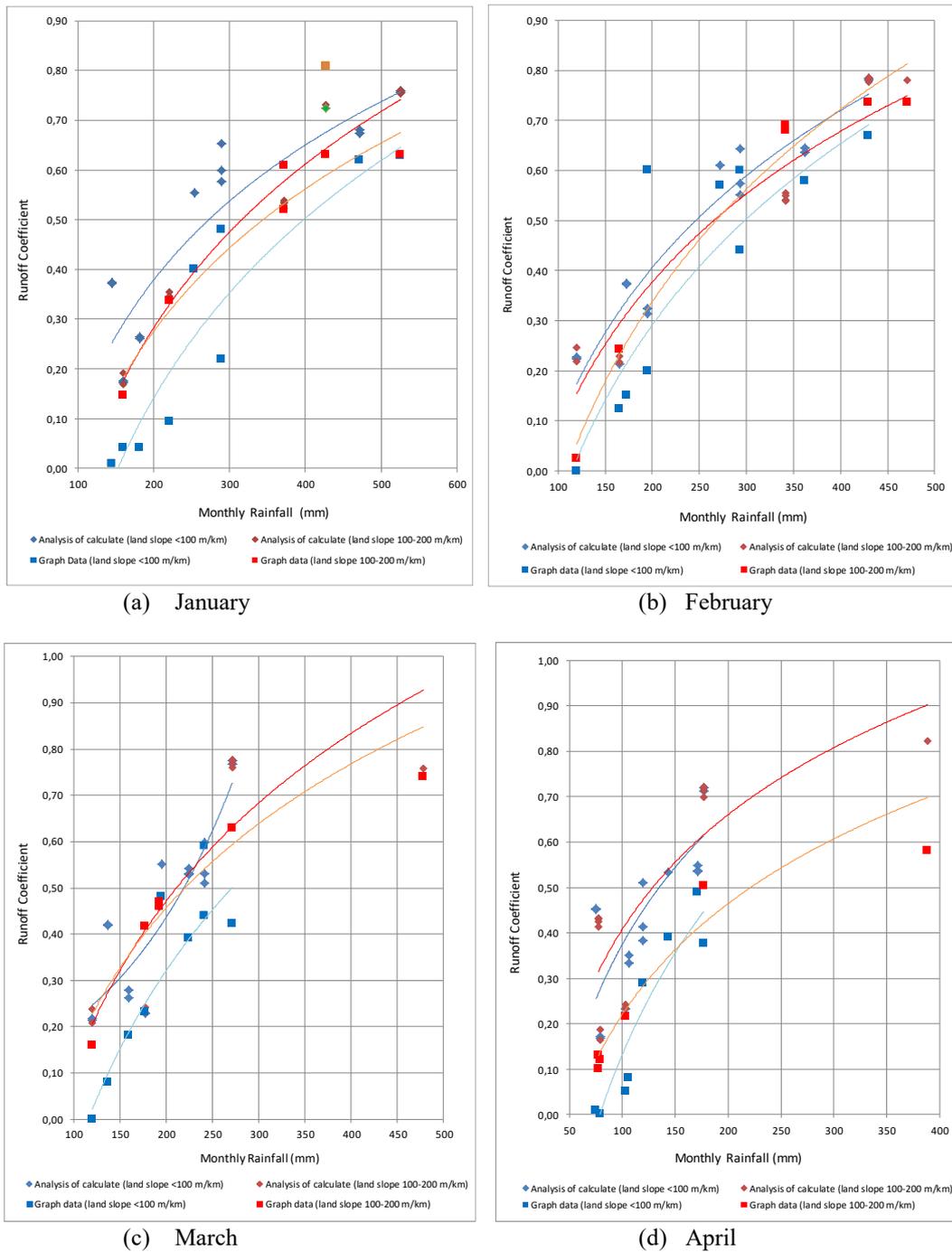


Figure 4. Graph high of monthly rainfall and slope of the land against runoff coefficient in the western part of Flores Island

Based on Figure 4 can be seen that the value of runoff coefficient in January and February has a tendency higher than March - April. In addition, the value of runoff coefficient is evenly distributed at the height of monthly rainfall than in March - April.

The difference value of calculation in January - April has ranging 10 - 20% higher than graph data from Research and Development Centre for Water Resources [7]. This is due to the transfer of land functions from forests into gardens or housing that make a lack of the catchment areas and decreasing

green land cover. It is resulting in greater surface runoff when it rains. Land-use and land-cover changes may have direct impacts on the hydrological cycle: they can cause floods, droughts, and changes in river and groundwater regimes [12].

4. Conclusions

The value of runoff coefficient by analysis for 30 retention ponds in Flores Island is as follows: a) West Flores (West Manggarai, Manggarai, East Manggarai, Ngada and Nagekeo) have the average of runoff coefficient value 0.12 – 0.61. The highest average of runoff coefficient value in Manggarai, Ngada, and Nagekeo. While other region has a low runoff coefficient value; b) East Flores (Ende, Sikka, and East Flores) have the average of runoff coefficient value 0.21 – 0.31. The value of runoff coefficient in this region lower because the eastern regions in Flores of the climate are drier so that the soil characteristic is more porous.

The runoff coefficient value based on analysis of calculate tends to be higher when compared with graph data from Research and Development Centre for Water Resources. This is due to the transfer of land functions or change land covering in catchment area. It is resulting in greater surface runoff when it rains.

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